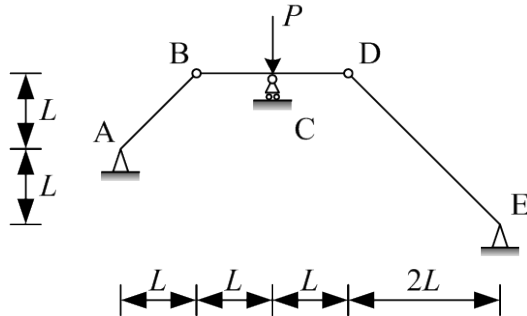
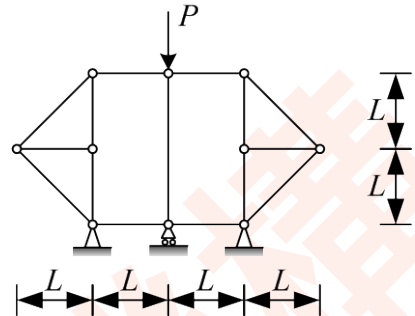


100 年鐵路人員特別考試高員三級結構學參考解答

一、請判別圖一(a)及圖一(b)中之結構。有小圈圈處代表鉸接點，否則為剛接。它們是穩定結構嗎？請說明你的判斷邏輯，否則不予計分；如果是穩定結構，它的超靜定次數 (static indeterminacy) 為若干？請說明你的判斷邏輯，否則不予計分，並求出應有之反力。(20 分)



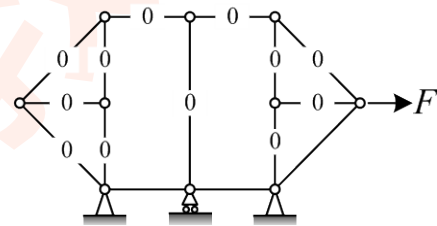
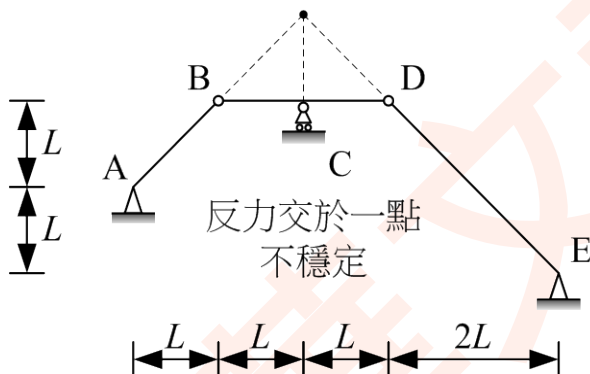
圖一(a)



圖一(b)

(100 鐵路高員-結構學 #1)

【參考解答】

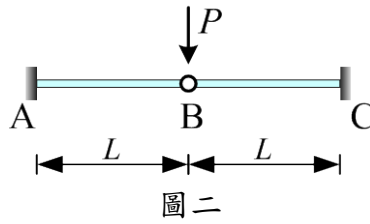


如圖所示，在該位置施加一水平力F 結構無法平衡，故為不穩定結構

【說明】

- (1)在圖一(a)結構中，當外力作用在 BCD 桿件時，二力桿 AB、DE 與支承 C 的反力會交交於一點，形成幾何不穩定結構系統。題目所給定之 P 恰好作用在反力交點上，剛好形成『不穩定平衡』。
- (2)在圖一(b)結構中，利用『任意載重法』加載並消除零桿後，會發現節點無法滿足靜平衡方程式，故為一不穩定結構系統。題目所給定之 P 恰好可形成『不穩定平衡』
- (3)一個穩定結構必須在任意載重下皆為穩定平衡，而不是在特定載重下維持穩定平衡。

二、設有一如圖二所示之複合梁 ABC，A、C 兩點為固定端，B 點為鉸接，梁之斷面彎曲剛度為 EI，B 點承受集中載重 P。試用共軛梁法 (conjugated beam method) 求 B 點之垂直變位 δ_B 及相對轉角 θ_B 。(25 分)



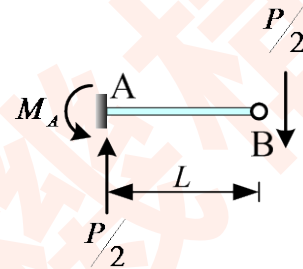
(100 鐵路高員-結構學 #2)

【參考解答】

(1) 結構內力分析

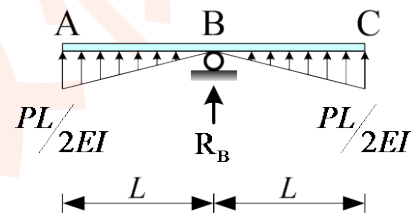
結構為對稱，A、B 兩固端支承反力為 $\frac{1}{2}P$

$$\text{取 AB 自由體：}\Sigma_B = 0 \Rightarrow \frac{P}{2} \times L = M_A = \frac{PL}{2}$$

(2) 繪製共軛梁圖，計算 $\overline{R_B}$

$$\Sigma F_y = 0 \Rightarrow 2 \left(\frac{1}{2} \times \frac{PL}{2EI} \times L \right) = \overline{R_B}$$

$$\therefore \overline{R_B} = \frac{PL^2}{2EI} \quad (\downarrow)$$

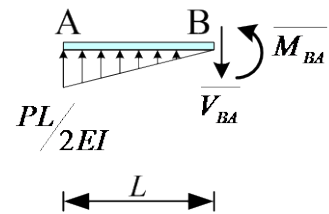
(3) 取共軛梁之 AB 段自由體計算 $\overline{V_{BA}}$ ， $\overline{M_{BA}}$

$$\Sigma F_y = 0 \Rightarrow \frac{1}{2} \times \frac{PL}{2EI} \times L = \overline{V_{BA}}$$

$$\therefore \overline{V_{BA}} = \frac{PL^2}{4EI} \quad (\downarrow)$$

$$\Sigma M_B = 0 \Rightarrow \left(\frac{1}{2} \times \frac{PL}{2EI} \times L \right) \times \frac{2}{3} L = \overline{M_{BA}}$$

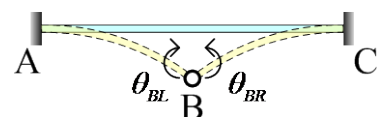
$$\therefore \overline{M_{BA}} = \frac{PL^3}{6EI} \quad (\circlearrowleft)$$

(4) 求解 δ_B

$$\delta_B = \overline{M_{BA}} = \frac{PL^3}{6EI} \quad (\downarrow)$$

(5) 求解 B 點相對轉角 θ_B

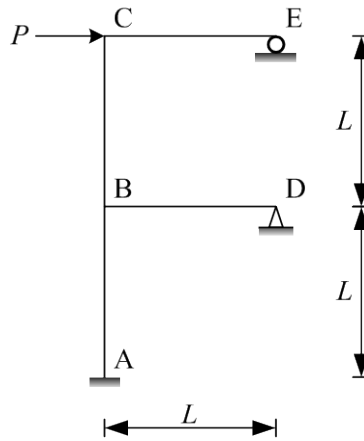
$$\theta_B = \overline{R_B} = \frac{PL^2}{2EI} \quad (\curvearrowright \cdot \curvearrowright)$$



$$\theta_{BL} = \frac{PL^2}{4EI} \quad (\circlearrowleft)$$

$$\theta_{BR} = \frac{PL^2}{4EI} \quad (\circlearrowleft)$$

三、設有如圖三之剛架，各桿斷面彎曲剛度均為 EI 。試用傾角變位法公式(slope-deflection equation)解此剛架，且繪此剛架之軸向力圖 (N-dia.)、剪力圖 (V-dia.)、彎矩圖 (M-dia.) 及彈性變形曲線。(30 分)



圖三

(100 鐵路高員-結構學 #3)

【參考解答】

(1) 桿件勁度比

$$k_{AB} : k_{BD} : k_{BC} : k_{CE} = \frac{2EI}{L} : \frac{2EI}{L} : \frac{2EI}{L} : \frac{2EI}{L} = 1:1:1:1$$

(2) 固端彎矩

無

(3) 自由度

$$\theta_B, \theta_C, R_{BC} = \frac{(\Delta_C)_h}{L} = R$$

(4) 傾角變位式

$$M_{AB} = 1 \cdot (\theta_B)$$

$$M_{BA} = 1 \cdot (2\theta_B)$$

$$M_{BD} = 1 \cdot (1.5\theta_B)$$

$$M_{BC} = 1 \cdot (2\theta_B + \theta_C - 3R)$$

$$M_{CB} = 1 \cdot (\theta_B + 2\theta_C - 3R)$$

$$M_{CE} = 1 \cdot (1.5\theta_C)$$

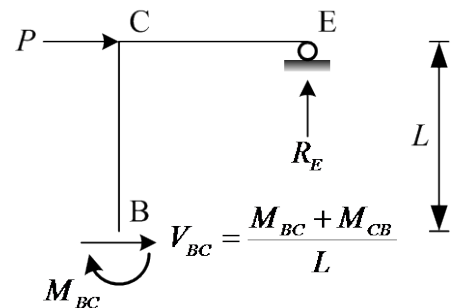
(5) 平衡方程式

$$\begin{aligned} \Sigma M_B = 0 &\Rightarrow M_{BA} + M_{BD} + M_{BC} = 0 \\ &\Rightarrow 5.5\theta_B + \theta_C - 3R = 0 \dots\dots\dots ① \end{aligned}$$

$$\begin{aligned} \Sigma M_C = 0 &\Rightarrow M_{BC} + M_{CE} = 0 \\ &\Rightarrow \theta_B + 3.5\theta_C - 3R = 0 \dots\dots\dots ② \end{aligned}$$

取 BCE 自由體

$$\begin{aligned} \Sigma F_x = 0 &\Rightarrow P + V_{BC} = 0 \\ &\Rightarrow 3\theta_B + 3\theta_C - 6R = -PL \dots\dots\dots ③ \end{aligned}$$

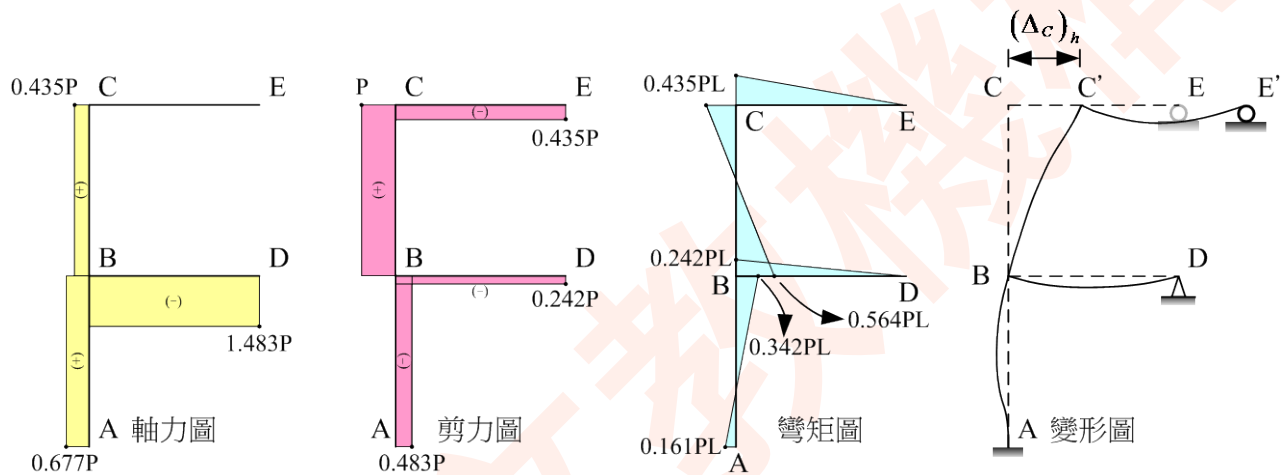


聯立①②③可解得： $\theta_B = 0.161PL$ ， $\theta_C = 0.290PL$ ， $R = 0.392PL$

將 θ_B 、 θ_C 、 R 帶回傾角變位式，可得各桿端彎矩

$$\Rightarrow \begin{cases} M_{AB} = 0.161PL \\ M_{BA} = 0.322PL \\ M_{BD} = 0.242PL \\ M_{BC} = -0.564PL \\ M_{CB} = -0.435PL \\ M_{CE} = 0.435PL \end{cases}$$

(6)繪製結構軸力圖、剪力圖、彎矩圖及彈性變形曲線

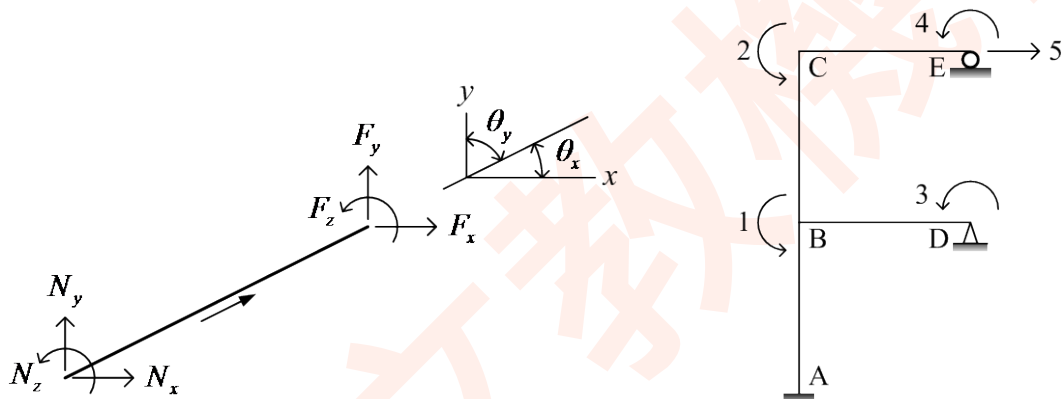


四、設有如圖三所示之剛架（第三題中之剛架），試求對應於圖四中所標示之五個自由度之勁度矩陣 $K_{5 \times 5}$ 。（提示：桿件勁度矩陣， k 。）（25 分）

桿件勁度矩陣， k ：

$$k = \begin{matrix} & \begin{matrix} N_x & N_y & N_z & F_x & F_y & F_z \end{matrix} \\ \begin{matrix} N_x \\ N_y \\ N_z \\ F_x \\ F_y \\ F_z \end{matrix} & \begin{bmatrix} \left(\frac{AE}{L}\lambda_x^2 + \frac{12EI}{L^3}\lambda_y^2\right) & \left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & -\frac{6EI}{L^2}\lambda_y & -\left(\frac{AE}{L}\lambda_x^2 + \frac{12EI}{L^3}\lambda_y^2\right) & -\left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & -\frac{6EI}{L^2}\lambda_y \\ \left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & \left(\frac{AE}{L}\lambda_y^2 + \frac{12EI}{L^3}\lambda_x^2\right) & \frac{6EI}{L^2}\lambda_x & -\left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & -\left(\frac{AE}{L}\lambda_y^2 + \frac{12EI}{L^3}\lambda_x^2\right) & \frac{6EI}{L^2}\lambda_x \\ -\frac{6EI}{L^2}\lambda_y & \frac{6EI}{L^2}\lambda_x & \frac{4EI}{L} & \frac{6EI}{L^2}\lambda_y & -\frac{6EI}{L^2}\lambda_x & \frac{2EI}{L} \\ -\left(\frac{AE}{L}\lambda_x^2 + \frac{12EI}{L^3}\lambda_y^2\right) & -\left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & \frac{6EI}{L^2}\lambda_y & \left(\frac{AE}{L}\lambda_x^2 + \frac{12EI}{L^3}\lambda_y^2\right) & \left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & \frac{6EI}{L^2}\lambda_y \\ -\left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & -\left(\frac{AE}{L}\lambda_y^2 + \frac{12EI}{L^3}\lambda_x^2\right) & -\frac{6EI}{L^2}\lambda_x & \left(\frac{AE}{L} - \frac{12EI}{L^3}\right)\lambda_x\lambda_y & \left(\frac{AE}{L}\lambda_y^2 + \frac{12EI}{L^3}\lambda_x^2\right) & -\frac{6EI}{L^2}\lambda_x \\ -\frac{6EI}{L^2}\lambda_y & \frac{6EI}{L^2}\lambda_x & \frac{2EI}{L} & \frac{6EI}{L^2}\lambda_y & -\frac{6EI}{L^2}\lambda_x & \frac{4EI}{L} \end{bmatrix} \end{matrix}$$

$$\lambda_x = \cos \theta_x, \lambda_y = \cos \theta_y$$



圖四

(100 鐵路高員-結構學 #4)

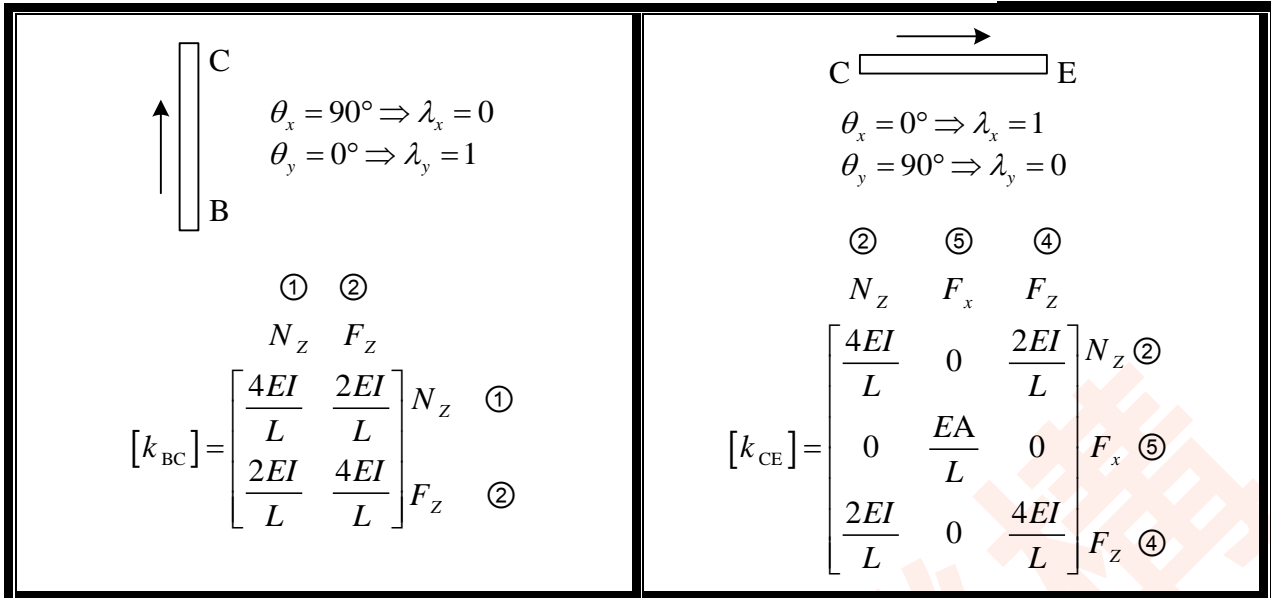
【參考解答】

$$\begin{matrix} \uparrow \\ \text{B} \\ \text{A} \end{matrix} \quad \begin{matrix} \theta_x = 90^\circ \Rightarrow \lambda_x = 0 \\ \theta_y = 0^\circ \Rightarrow \lambda_y = 1 \end{matrix}$$

$$[k_{AB}] = \begin{bmatrix} 4EI \\ L \end{bmatrix} F_z \quad \text{①}$$

$$\begin{matrix} \rightarrow \\ \text{B} \quad \text{D} \end{matrix} \quad \begin{matrix} \theta_x = 0^\circ \Rightarrow \lambda_x = 1 \\ \theta_y = 90^\circ \Rightarrow \lambda_y = 0 \end{matrix}$$

$$[k_{BD}] = \begin{bmatrix} 4EI & 2EI \\ L & L \\ 2EI & 4EI \\ L & L \end{bmatrix} \begin{matrix} N_z & F_z \\ N_z & F_z \end{matrix} \quad \begin{matrix} \text{①} \\ \text{③} \end{matrix}$$



$$[K] = \frac{E}{L} \begin{bmatrix} 4I+4I+4I & 2I & 2I & 0 & 0 \\ 2I & 4I+4I & 0 & 2I & 0 \\ 2I & 0 & 4I & 0 & 0 \\ 0 & 2I & 0 & 4I & 0 \\ 0 & 0 & 0 & 0 & A \end{bmatrix}$$

$$= \frac{E}{L} \begin{bmatrix} 12I & 2I & 2I & 0 & 0 \\ 2I & 8I & 0 & 2I & 0 \\ 2I & 0 & 4I & 0 & 0 \\ 0 & 2I & 0 & 4I & 0 \\ 0 & 0 & 0 & 0 & A \end{bmatrix}$$